

## TR-20 Structure and Operation

Technical Release 20: Computer Program for Project Formulation Hydrology (TR-20) (SCS, 1983) is a hydrologic model for simulation of direct runoff hydrographs resulting from natural or synthetic rainfall occurring over a watershed. The program can be used to model complex watersheds with multiple subareas, channel reaches, and reservoirs. Hydrographs are generated for subwatersheds, combined or separated at confluence points, and routed through downstream reaches or structures.

### Conceptual Description

The program is designed to simulate the rainfall-runoff process for a watershed. Figure 2-1 depicts the rainfall-runoff process used by TR-20. The unit hydrograph is the transfer function used by TR-20 to transform the rainfall excess into direct runoff. The rainfall hyetograph is convolved with the unit hydrograph to produce the direct runoff hydrograph. This process is explained below.

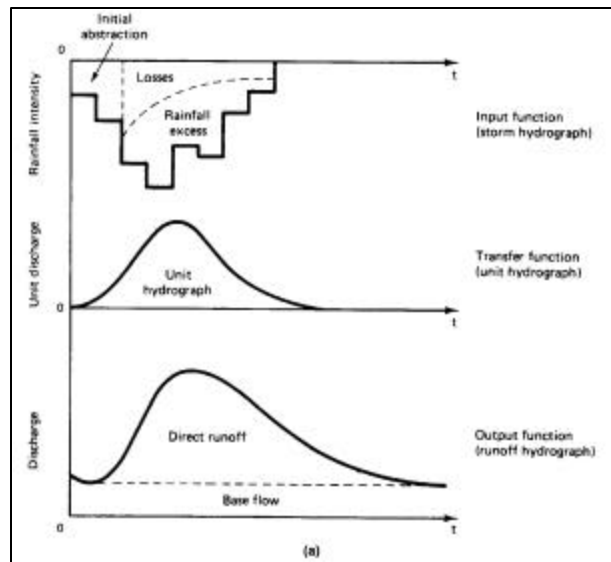


Figure 2-1: Rainfall-runoff process (McCuen, 1998).

For a specified rainfall depth and distribution, a hydrograph is developed for each subwatershed given the drainage area, time of concentration, and curve number as input. The runoff resulting from a given rainfall depth is computed from the NRCS rainfall-runoff equation:

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S}$$

where P is the precipitation in inches, Q is the runoff in inches, and S is the potential storage in inches, given by:

$$S = \frac{1000}{CN} - 10$$

where CN is the runoff curve number. The time to peak for the direct runoff hydrograph is equal to two-thirds the time of concentration. The peak discharge of the unit hydrograph, the SCS dimensionless unit hydrograph in this case, is computed using the equation:

$$q_p = \frac{484AQ}{t_p} = \frac{726AQ}{t_c}$$

where  $t_p$  is the time to peak in hours,  $t_c$  is the time of concentration in hours, Q is the runoff of one inch for the unit hydrograph, and A is the drainage area in square miles.

The ordinates of the rainfall hyetograph are multiplied, translated, and added in time with the ordinates of the unit hydrograph to form the direct runoff hydrograph for each subwatershed. This process is called convolution. The value 484 in the above equation is called the peak rate factor and represents an empirical constant.

Hydrographs generated from upstream subwatersheds can be routed through channel reaches using the Modified Att-Kin (Attenuation – Kinematic) method. For a given stream reach, the routing procedure translates the upstream hydrograph along its length and attenuates the peak of the hydrograph to account for storage in the channel. Hydrographs can also be routed through reservoirs or other storage structures, which TR-20 uses the storage-indication method to simulate.

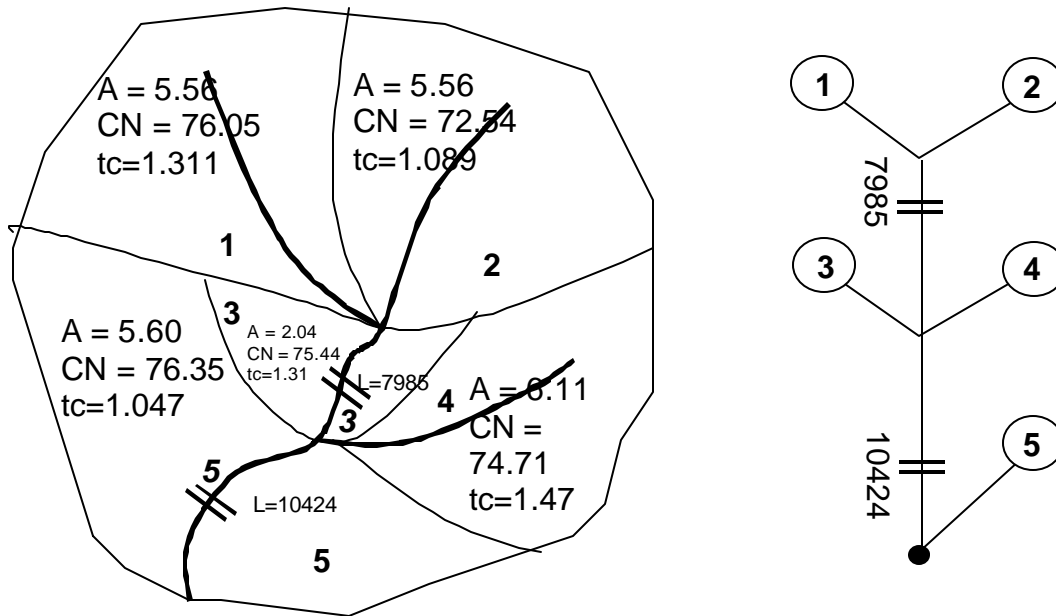
### **Simplified Example**

The best way to illustrate the overall process of developing a TR-20 model is to walk through a simplified example. The example that follows gives a 24.9 mi<sup>2</sup> watershed divided into five subwatersheds. It contains two reach routings in the lower portion of the main channel. At the outlet, the peak discharge resulting from 4.25-in of rainfall is simulated for the 24-hr Type II storm.

In order to develop a simulated runoff hydrograph for each subwatershed, a minimum amount of information is required. The area, time of concentration, and curve number must be specified for each subwatershed. In order to route upstream hydrographs to downstream locations, a representative cross section is needed from which a stage-discharge-end area relationship can be produced.

## Watershed Configuration

Consider the following arrangement of subwatersheds for the example TR-20 model:



Hydrographs are generated for each of the subareas and are combined *in time* at the junction points. The difference in the timing of runoff for each subarea (i.e. time of concentration) controls how the peak discharge will be affected when the hydrographs are combined. If the times are very close, the times to peak of each of the hydrographs will be similar and a larger peak discharge will result. If the times are substantially different, the peaks will not coincide and a reduced peak will result. The consideration of increased peak does not ignore volume considerations. Regardless of the timing of runoff, the volume of runoff generated by a real or synthetic rainfall distribution will eventually be translated in time through the watershed.

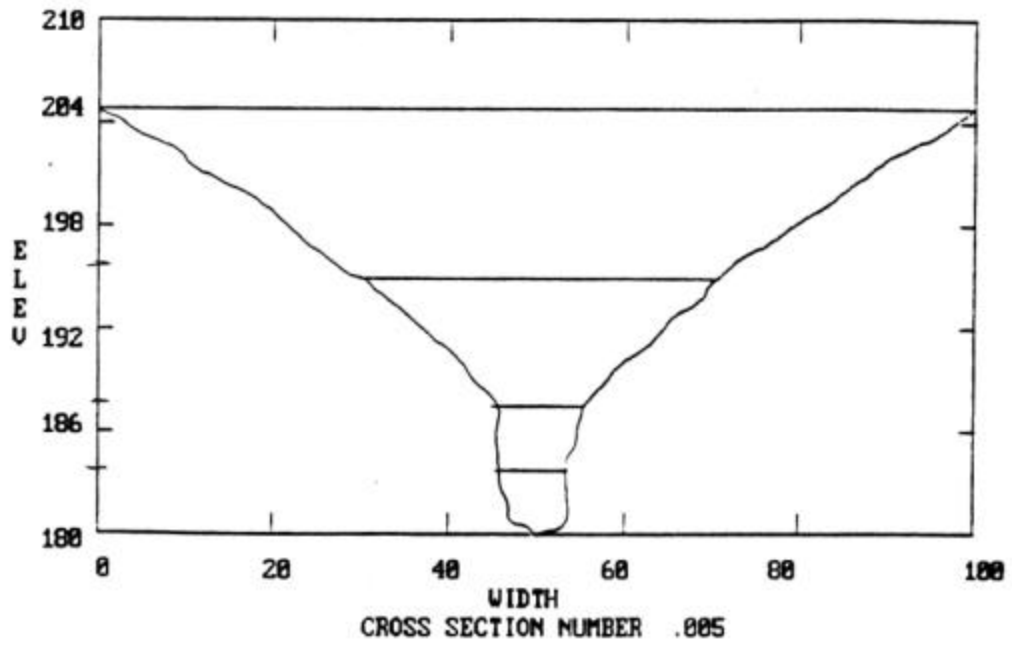
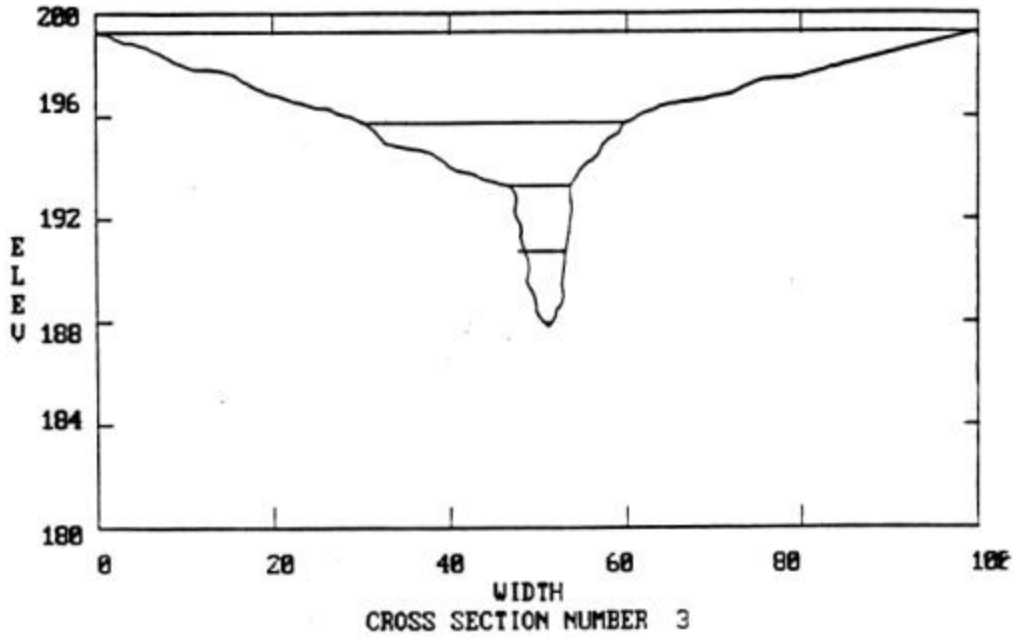
## Cross Sections

Cross sections control not only translation of upstream hydrographs but also attenuation. The attenuation is based on the storage available to runoff while in the channel/floodplain.

Two cross sections are identified in the watershed that will be used to route hydrographs. Cross Section 3 (L = 7985 ft.) will be used to route the combined hydrograph from subwatersheds 1 and 2. Cross Section 5 (L = 10424 ft.) will be used to route the combine hydrograph resulting from subwatersheds 3 and 4 and the routed hydrograph through Cross Section 3.

Representative cross sectional geometry was determined for each of the sections for the channel and overbank. Additionally, roughness characteristics, given in terms of

Manning roughness coefficients were determined for Section 3 (channel n: 0.035, left n: 0.07, and right n: 0.09) and for Section 5 (channel n: 0.035, left n: 0.07, right n: 0.09). The sections are shown below:



### ***Rating Table Calculation***

For each of the cross sections shown above, a stage-discharge-end area relationship must be developed. This information is used by the TR-20 model in the Modified-Att-Kin reach routing procedure. At incremental depths, values of stage, discharge and end area are computed. The relationship below can be used to develop the reach rating table for a complex cross section involving a center channel, left and right overbank.

$$Q = \sum \Delta Q = A_L V_L + A_c V_c + A_R V_R$$

where V is the velocity from Manning's Equation:

$$V = \frac{1.49}{n} R^{\frac{2}{3}} S^{\frac{1}{2}}$$





8				193.29	85.823	24.671
8				195.78	247.063	70.625
8				199.28	923.957	297.058
9	ENDTBL					
2	XSECTN	005		1.0	187.39	
8				180.	0.000	0.000
8				183.69	95.543	21.501
8				187.39	333.701	55.523
8				194.86	1730.803	242.722
8				204.79	7160.793	935.737
9	ENDTBL					
6	RUNOFF	1	1	5.5599	76.048	1.311
6	RUNOFF	1	2	5.5599	72.541	1.089
6	ADDHYD	4	9 1 2 3			
6	RUNOFF	1	3	2.0411	75.444	1.314
6	REACH	3	8 3 2	7985.		
6	ADDHYD	4	9 1 2 3			
6	RUNOFF	1	4	6.1100	74.710	1.470
6	ADDHYD	4	7 4 3 2			
6	REACH	3	6 2 1	10424.		
6	RUNOFF	1	5	5.6000	76.350	1.047
6	ADDHYD	4	10 1 5 4			1
	ENDATA					
7	INCREM	6		0.1		
7	COMPUT	7 001	010	0.0	4.25	1.02 2 1 1
	ENDCMP	1				
	ENDJOB	2				

### Output File.

TR-20 creates an output file which reports the desired output options (hydrographs, peaks, elevations, volumes) at desired locations within the watershed. The output file for the sample watershed is shown below:

```

1
TR20 -----
--- SCS -
                                OUTLET OF BASIN AT WATERSHED #5
VERSION
  03/10/**
2.04TEST
  13:10:42                PASS  1  JOB NO.  1
PAGE  1

OPERATION ADDHYD  XSECTION  10
          INPUT HYDROGRAPHS 1,5  OUTPUT HYDROGRAPH 4

          PEAK TIME(HRS)                PEAK DISCHARGE(CFS)                PEAK
ELEVATION(FEET)
          13.30                          6818.4                          (NULL)

          RUNOFF ABOVE BASEFLOW (BASEFLOW = .00 CFS)
          1.85 WATERSHED INCHES;  29746 CFS-HRS;  2458.2
ACRE-FEET.
1

```



TR20 -----  
--- SCS -

OUTLET OF BASIN AT WATERSHED #5

VERSION  
03/10/\*\*  
2.04TEST  
13:10:42

PASS 2 JOB NO. 1